

1 **The effectiveness of using Raw Sewage Sludge (RSS) as a water**
2 **replacement in cement mortar mixes containing Unprocessed Fly Ash (u-**
3 **FA).**

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26 **Abstract**

27 The performance of two groups of mortar mixes containing Unprocessed Fly Ash (u-FA) with
28 either Raw Sewage Sludge (RSS) or water was examined. Both groups included four mortar
29 mixes containing Portland cement, sand, u-FA. Group 1 used RSS as a water replacement
30 and Group 2 used water. Cement was replaced with 0, 10, 20 and 30% u-FA of total binder
31 weight and one Liquid/Binder ratio of 0.8 was used. Mortar mixes were tested for their
32 flowability, Total Water Absorption (TWA), Ultrasonic Pulse Velocity (UPV), compressive
33 strength and drying shrinkage. The outcomes of the investigation were encouraging in that
34 cement-based materials containing RSS demonstrated good engineering properties in
35 comparison to the control mixes. The inclusion of u-FA significantly reduced flowability;
36 however improved long-term compressive strength for both groups. The greatest
37 compressive strength was recorded for the mixes with 10-20% u-FA replacement.

38 **Keywords**

39 Raw Sewage Sludge (RSS), Unprocessed Fly Ash (u-FA), Sustainable Construction Materials.

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48 **1 Introduction**

49 **1.1 Sewage sludge**

50 Raw Sewage Sludge (RSS^{*}) is a residual stream of suspended or dissolved organic and
51 inorganic materials that result from the treatment processes of municipal wastewaters. RSS
52 is usually in the form of liquid or semisolid liquid that typically contains from 2 to 8 percent
53 solids by weight, depending on the operation and processes applied. In wastewater
54 treatment plants, RSS is mainly collected from primary settlement tanks, which are large
55 round or rectangular in shape and allow heavier particles to settle to the bottom and later
56 swept by scrapers to a submerged outlet. The settled stream is pumped, in the form of thick
57 slurry, to the sludge storage and treatment unit for further processing. RSS may also be
58 collected from secondary and tertiary settlement tanks [1].

59 At sewage sludge storage and treatment units, further biological, chemical and physical
60 processes are applied to reduce the water content and eliminate potential associated
61 hazards of RSS. Hazards include high heavy metal contents, presence of dangerous
62 pathogens and risks associated with the biodegradation of organic matters (production of
63 flammable gases and unpleasant odours). Treatment processes include preliminary
64 operations, thickening, stabilisation, conditioning, dewatering, heat drying and other
65 processing and thermal reduction [1].

66 There are approximately 35 million tonnes of RSS produced in the UK each year. These
67 quantities are reduced to 25 million tonnes per year by applying further on site physical and
68 chemical processes [2]. In 2010, 1.41 million tons of dry solids were produced from sewage
69 sludge in England and Wales [3].

* Abbreviations: RSS is Raw Sewage Sludge, u-FA is Unprocessed Fly Ash, TWA is Total Water Absorption, and UPV is Ultrasonic Pulse Velocity.

70 Prior to the implementation of the European Union Urban Waste Water Treatment Directive
71 (91/271/EEC) in 31 Dec 1998, around a quarter of the sewage sludge produced in the UK
72 was either discharged to surface waters via pipes or disposed from ships at sea [3]. The
73 discontinuation of this route, together with the stringent standards required by the
74 European Waste Water Directive, generated excessive quantities of sewage sludge, adding
75 greater challenges for environmental agencies and local authorities. Since then, the
76 traditional re-use and disposal methods have had to be replaced by effective alternatives to
77 improve waste management practices currently in place. Alternative methods include the
78 utilisation of sewage sludge products in the construction industry for the production of
79 sustainable construction materials [4].

80 Sewage sludge products have been recently introduced as sustainable alternatives to the
81 traditional raw ingredients used in the construction industry. These include dewatered
82 sewage sludge, dry sewage sludge and incinerated sewage sludge ash. These materials were
83 used in different construction applications including the production of cement-based
84 materials [5-10], ceramic products such as ceramics tile bodies [11-18], lightweight
85 construction materials [19-23], soil stabilisation [24-26], and other civil engineering
86 applications such as wastewater treatment [27] and landfill lining [28].

87 **1.2 Unprocessed Fly Ash**

88 In addition to the problems caused by excessive RSS production in the UK, the power
89 generation industry also produces vast quantities of fly ash from burning coal. In the UK,
90 there is approximately 5,300,000 tonnes of fly ash produced annually [29]. Unprocessed Fly
91 Ash (u-FA) is not suitable for use in construction applications due to its high carbon content
92 and large particle size [30-33]. Therefore there is a requirement for the u-FA to be treated
93 and classified to meet the requirements of the European Standards, and this process often

94 involves a series of costly and energy consuming mechanical and physical applications.
 95 Although there is very limited information about the utilisation of u-FA, the current
 96 literature suggests that incorporating this in cement-based materials would improve its
 97 mechanical and durability properties [34, 35].

98 2 Experimental

99 2.1 Materials and mixing proportion

100 In this experimental work, the performance of two groups of mortar mixes containing 0-30%
 101 u-FA of total binder weight with either RSS or water were examined. Group 1 and Group 2
 102 used the same composition of Portland Cement, sand and u-FA. Group 1 used RSS as a
 103 water replacement whereas Group 2 used water and was considered as the control (Table
 104 1). Mortar mixes were tested for their fresh and engineering properties including
 105 flowability, TWA, UPV, compressive strength and drying shrinkage.

106 **Table 1: Mixing composition.**

Group	Mix	Binder		Sand	Liquid/Binder	Liquid type
		Cement	u-FA			
1	MR1	1	0	4.5	0.8	RSS
	MR2	0.9	0.1			
	MR3	0.8	0.2			
	MR4	0.7	0.3			
2	M1	1	0	4.5	0.8	Water
	M2	0.9	0.1			
	M3	0.8	0.2			
	M4	0.7	0.3			

107
 108 The cement used throughout the experimental programme was Portland Cement that
 109 complies with the requirements of BS EN 197-1:2000 type CEM I Portland cement strength
 110 class 42.5 [36]. The fly ash used in this experimental work was u-FA that was collected from
 111 a coal power station in the UK. The RSS sample was collected from a Sewage Treatment
 112 Works in the West Midlands, UK in the form of thick slurry containing 97.5% liquid of total
 113 weight (Figure 1).

114 Table 2 shows the physical and chemical properties of u-FA and RSS used, and Figure 2
 115 shows the particle size distribution of u-FA. 0.5% Hydrated Lime of total RSS weight was
 116 added for partial treatment of RSS to eliminate pathogens by raising the pH level >12. The
 117 amount of the hydrated lime added was estimated based on recommendations made by the
 118 British Lime Association [37]. The sand used throughout this work was size 0/4 that complies
 119 with the requirements of BS EN 12620:2002+A1:2008 category G_F85 [38], and fineness
 120 content category 1 [39]. The mixing water used for the control was tap water that complies
 121 with the requirements of BS EN 1008:2002 [40] and BS EN 206-1:2000 [41].



122
 123 **Figure 1: Raw Sewage Sludge sample.**

124 **Table 2: Physical and chemical properties of u-FA and RSS.**

Material	Property/element	Unit	Value	Techniques
u-FA	Moisture content	% weight	0.78	-
	Bulk density	Kg/m ³	442	BS EN 1097-3:1998 [42]
	Dry particle density	Kg/m ³	1824	100ml Pycnometer
	SiO ₂	% total weight	45.06	X-Ray Fluorescence (XRF)
	Al ₂ O ₃		16.94	
	Fe ₂ O ₃		9.04	
	CaO		1.96	
	K ₂ O		1.4	
	MgO		1.02	
	TiO ₂		0.71	
	Na ₂ O		0.34	
	P ₂ O ₅		0.19	
	BaO		0.08	
	ZrO ₂		0.07	
SrO	0.06			
MnO	0.05			
ZnO	0.04			

	Cr ₂ O ₃ CuO PbO		0.02 0.02 0.02	
	Loss on Ingestion (LOI)	% total weight	23	Thermogravimetry
RSS	Liquid content	%	97.5	Drying oven
	Density	Kg/m ³	1012	BS EN 1097-7:2008 [43]
	Chloride	ppm	32.19	Ion Chromatography System (ICS)
	Nitrate		2.94	
	Phosphate		1.38	
	Sulphate		23.93	
	Cr	ppm	1.19	Inductively Coupled Plasma (ICP)
	Cu		5.33	
	Ni		2.51	
	Sn		0.04	
Zn	19.08			
Mn	3.92			
Fe	147.72			
Al	77.83			
As	0.27			
Ba	7.55			
S	65.22			
P	200.83			
Na	199.65			
Mg	54.77			
K	121.05			
Ca	33793.35			
Dry solids of sewage sludge	Na ₂ O	% total weight	23.46	X-Ray Fluorescence (XRF)
	MgO		3.35	
	Al ₂ O ₃		2.53	
	SiO ₂		8.54	
	P ₂ O ₅		8.04	
	SO ₃		4.45	
	Cl		0.15	
	K ₂ O		0.58	
	CaO		33.78	
	TiO ₂		0.4	
	Fe ₂ O ₃		11.11	
	ZnO		0.26	

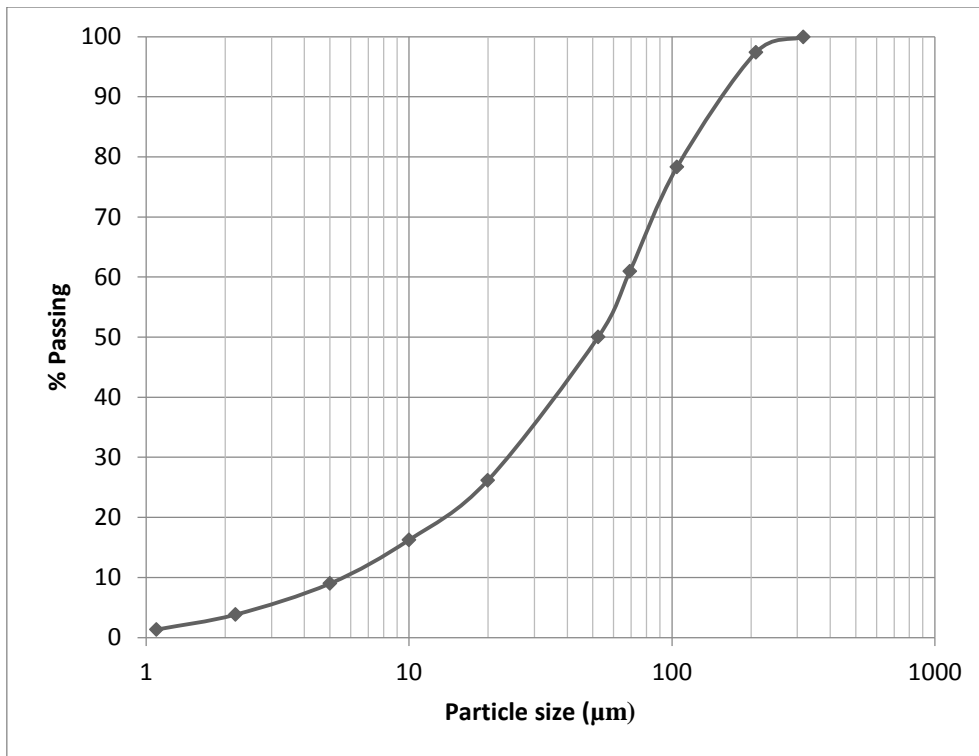


Figure 2: Particle size distribution of the u-FA sample.

2.2 Casting, curing and testing

Steel moulds of 50mm in size were used to prepare mortar specimens for the determination of Total Water Absorption (TWA), Ultrasonic Pulse Velocity (UPV) and compressive strength. For the determination of drying shrinkage, prisms of dimensions 40mm x 40mm x 160 mm in size were used. Cast specimens were covered with plastic sheets and placed in a room (temperature of $20^{\circ}\text{C} \pm 2^{\circ}\text{C}$) for 24 hours until demoulding. Thereafter, cubes were cured for 1, 7, 28, 90 and 365 days by wrapping them using cling film. The prisms were left exposed for 360 days during which regular shrinkage readings were taken using a dial gauge.

The flowability of fresh mixes was obtained using the flow table test that complies with the requirements of BS EN 1015-3, 1999 [44]. To determine the Total Water Absorption (TWA), cured specimens were placed in an electrical oven at 75°C until a constant weight was achieved. Thereafter, dried specimens were allowed to cool in a room (temperature of 20°C), and the mass was later recorded to the nearest 0.1g. Dried samples were submerged

140 in water until a constant weight was reached (weights were monitored at 24 hour intervals).
141 Prior to measuring the mass of the saturated samples, excess water was removed using
142 damp towels. Total Water Absorption was calculated using Equation 1, and the average of
143 three specimens was recorded to the nearest 0.1%.

$$144 \quad TWA = (m_s - m_d) * 100/m_d \quad \text{Equation 1}$$

145 Where

146 TWA is Total Water Absorption %;
147 m_s is mass of saturated samples, in g;
148 m_d is mass of dried samples, in g.

149 Ultrasonic Pulse Velocity (UPV) was obtained by measuring the time requirements for an
150 ultrasonic pulse to transmit through test specimens using Proceq Pundit Lab+ instrument.
151 The average of three specimens (six sides) was recorded to the nearest 1m/sec. For the
152 determination of compressive strength, the average of three cubes was recorded to the
153 nearest 0.1 MPa. Mortar samples were tested in accordance to ASTM C109/C109M-02 [45]
154 using SERCOMP7 hydraulic compressive strength machine with a loading rate of 2400 N/Sec.
155 Length change due to drying shrinkage was obtained by attaching two pairs of demec-studs
156 to the two sides of the prism that were cast against the steel mould (100mm between each
157 stud). Demec-studs were attached immediately after demoulding using conventional super
158 glue, and prisms were placed in a room (temperature of $20^{\circ}\text{C} \pm 2^{\circ}\text{C}$ and a relative humidity
159 of $50\% \pm 10\%$). Length change was monitored regularly using a digital dial gauge. The
160 average reading of three specimens (6 sides) was calculated using Equation 2 and was
161 recorded to the nearest 1μ strain.

$$\varepsilon = (L_2 - L_1) * 10^6/L_1 \quad \text{Equation 2}$$

162 Where

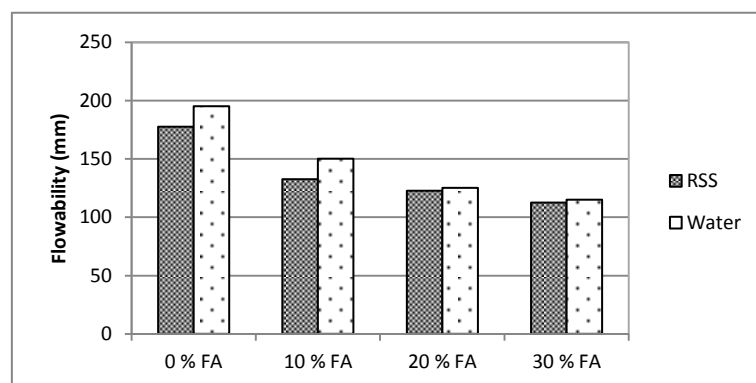
163 ε is strain, in Micro Strain;

164 L_2 is new length (new gauge reading), in mm;
165 L_1 is the original length (original gauge reading), in mm.

166 3 Results and discussion

167 3.1 Flowability

168 The flowability of the mortar mixes made using RSS or water and that also contained
169 different amounts of u-FA is shown in Figure 3. The flowability of mortar reduced when the
170 content of u-FA increased, and the lowest flowability of 113mm was recorded for the
171 mortar mix with RSS and 30% u-FA (MR4). For the control mixes (Group 2), the flowability
172 also decreased with the addition of u-FA and the lowest flowability of 115mm was recorded
173 for the mortar mix with 30% u-FA (M4). The reduction in flowability may be due to the high
174 unburned carbon content of the u-FA, which absorbs hydration water resulting in less
175 workability [46-49]. The flowability of the mortar mixes with RSS was comparatively less
176 than those for the mixes with water.

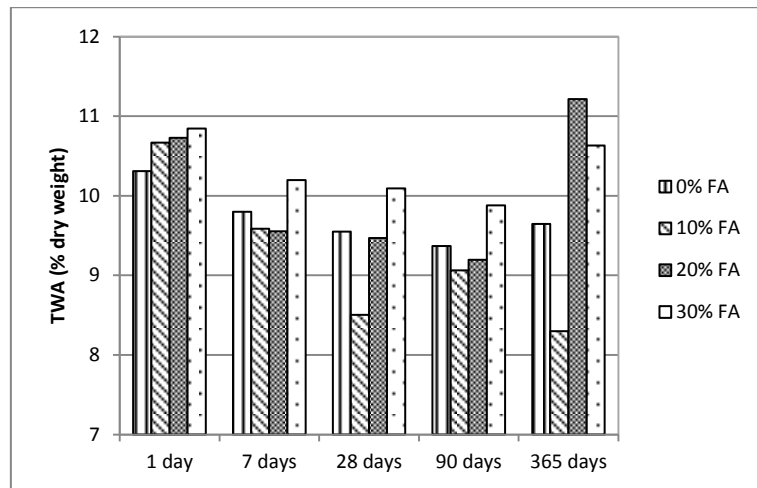


177
178 **Figure 3: Flowability of mortar mixes with RSS or water.**

179 3.2 Total Water Absorption (TWA)

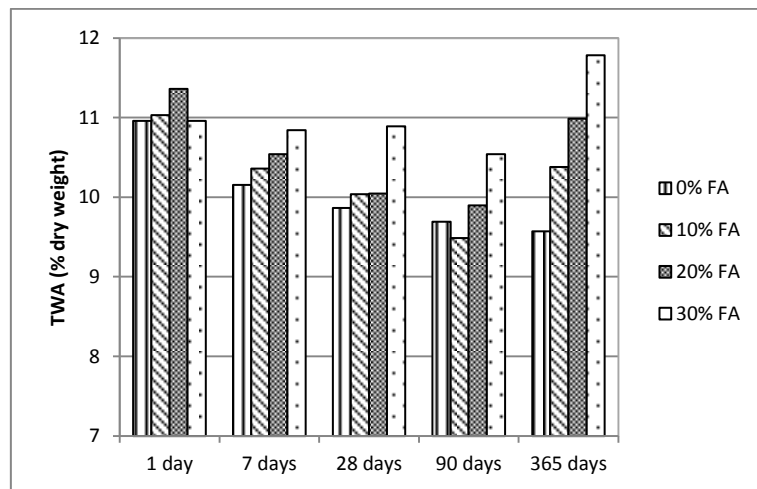
180 The TWA for the mortar mixes with RSS is presented in Figure 4. The results showed that
181 TWA decreased with curing age for all mixes up to 365 days, except for the mortar mix with
182 10% u-FA replacement. Figure 5 shows TWA results for the mortar mixes with water. The
183 results generally showed that TWA decreased with curing age for all mixes, as TWA
184 decreased with age up to 365 days. The irregularity in grading and particle shape of u-FA

185 influences the water absorption properties by generating additional voids in the produced
 186 mortar. Water absorption properties also depend on the LOI values, which is directly
 187 associated with the amount of porous carbon contained in u-FA [46-49].



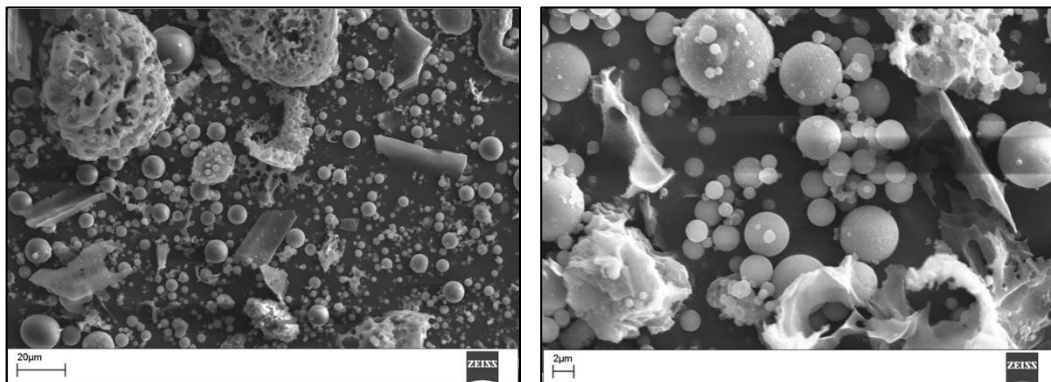
188
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Figure 4: TWA for the mortar mixes with RSS and different u-FA content.



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Figure 5: TWA for the mortar mixes with water and different u-FA content.



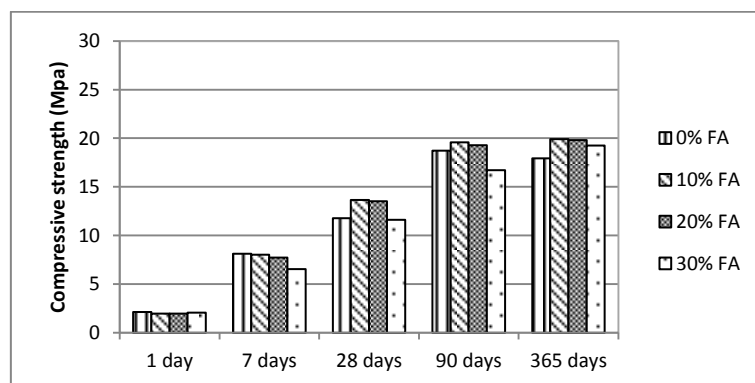
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Figure 6: Images of u-FA using Scanning Electron Microscopes Technology (SEM).

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195 3.3 Compressive strength

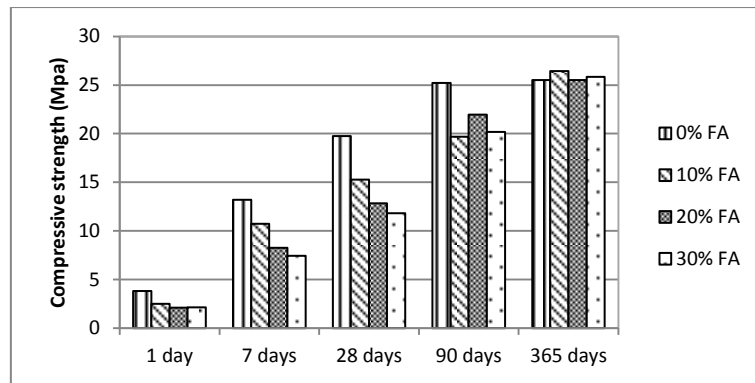
196 Figure 7 presents the compressive strength of the mortar mixes with RSS. At 1 and 7 days
197 the compressive strength generally decreased when u-FA was included. At later ages, the
198 result showed that the compressive strength improved when the content of u-FA increased,
199 and the greatest compressive strength was recorded for the mortar mixes with 10 and 20%
200 u-FA replacement. The results also showed that the addition of u-FA improved long-term
201 strength, and prevented the decline in compressive strength observed for the mortar mix
202 without u-FA at 365 days. The compressive strength for the mortar mixes with water is
203 presented in Figure 8. The Figure showed that the compressive strength at 1, 7, 28 and 90
204 days decreased when u-FA content increased, and the greatest compressive strength was
205 achieved for the mortar mix with 0% u-FA. At 365 days the results showed a significant
206 improvement in the compressive strength for all mixes that contained u-FA, and the
207 greatest compressive strength of 26.4 MPa was recorded for the mortar mix with 10% u-FA.
208 This may be due to the positive impact of the pozzolanic activities of fly ash particles on
209 long-term strength development [50-57]. The compressive strength of the mortar mixes
210 with RSS was noticeably less than that of the mixes with water.



211

212

Figure 7: Compressive strength of mortar mixes with RSS and different u-FA content.



213

214

Figure 8: Compressive strength of mortar mixes with water and different u-FA content.

215

3.4 Ultrasonic Pulse Velocity (UPV)

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The UPV of the mortar mixes with RSS is presented in Figure 9. The Figure shows that UPV

217

values increased with curing age up to 90 days. It also shows that the UPV values at earlier

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ages (1 and 7 days) generally decreased when the content of u-FA increased. At later ages

219

(28 and 90 days) no significant differences in UPV was observed, but some improvement in

220

the UPV was noted at 365 days when u-FA was increased up to 20% replacement. It was also

221

noted that the UPV values at 365 days were relatively less than those at 90 days, and this

222

may be associated with degradation process of the organic component in RSS. The UPV of

223

the mortar mixes with water is presented in Figure 10. The results showed that UPV values

224

at 1, 7 and 28 days decreased when u-FA content increased and the greatest UPV readings

225

were recorded for the mix with 0% u-FA. At later ages (90 and 365 days), the UPV values for

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the mortar mixes with u-FA were comparatively greater than those without, and the

227

greatest UPV values were recorded at 365 days. The results also showed that UPV values

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continued to increase with time up to 365 days except for the mix with 0% u-FA (an

229

anomalous result). This may be associated with the long-term strength development that

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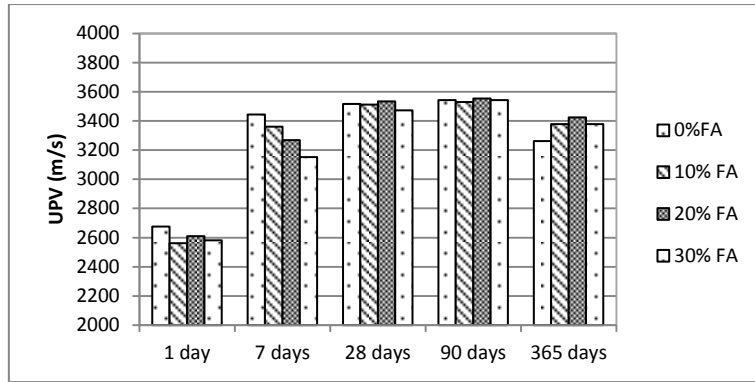
resulted from the inclusion of fly ash [50-53]. The UPV of the mortar mixes with water was

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generally higher than that of the mixes with RSS (except at 1 day for the mixes with 20 and

232

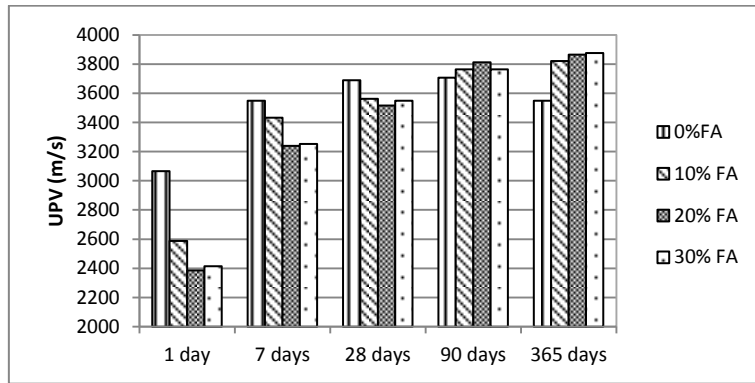
30%).



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Figure 9: UPV of the mortar mixes with RSS and different u-FA content.



235

236

Figure 10: UPV of the mortar mixes with water and different u-FA content.

237 3.5 Correlation

238 The relationship between the compressive strength and UPV is shown in Figure 11. The

239 Figure demonstrates a strong correlation between these two properties, and Equation 3 has

240 been developed to predict the compressive strength by using the non-destructive readings

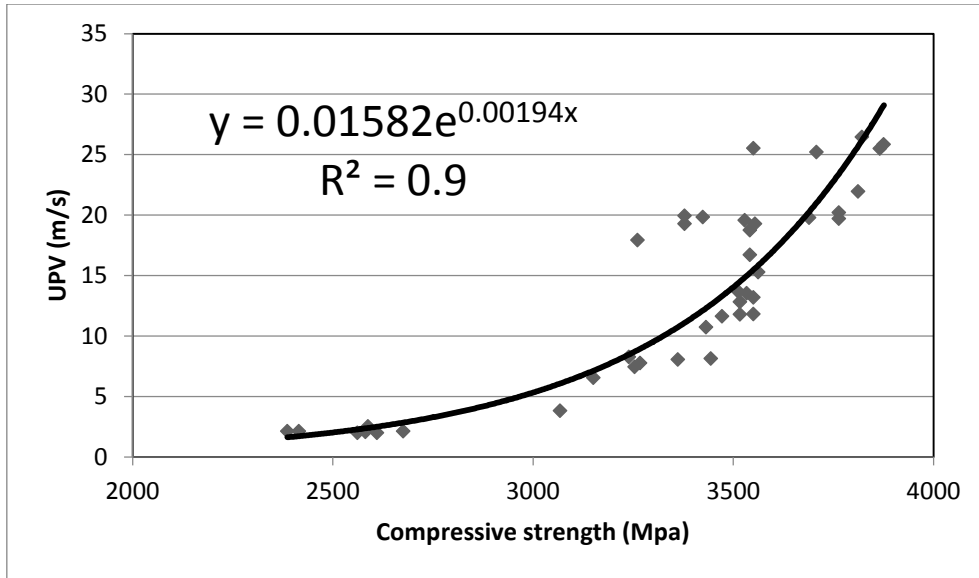
241 of the UPV test.

242 $y = 0.01582e^{0.00194x}$

Equation 3

243 Where:

244 y is UPV in m/s, and x is compressive strength in MPa



245

246

Figure 11: Correlation between compressive strength and UPV.

247 **3.6 Length change**

248 The length change of the mortar mixes with RSS and water is shown in Figure 12 and Figure

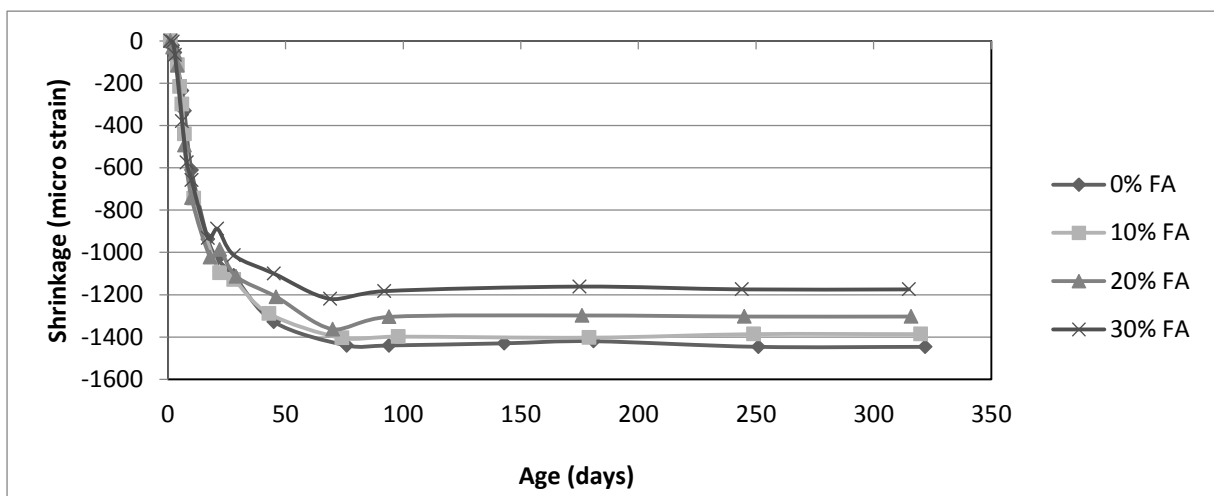
249 13 respectively. The results clearly showed that drying shrinkage decreased when u-FA

250 content increased, and the best results were observed when 30% u-FA was added.

251 Moreover, the drying shrinkage mostly occurred during the first 50-70 days, through which

252 no significant impact of the inclusion of u-FA on drying shrinkage was noted. The mixes with

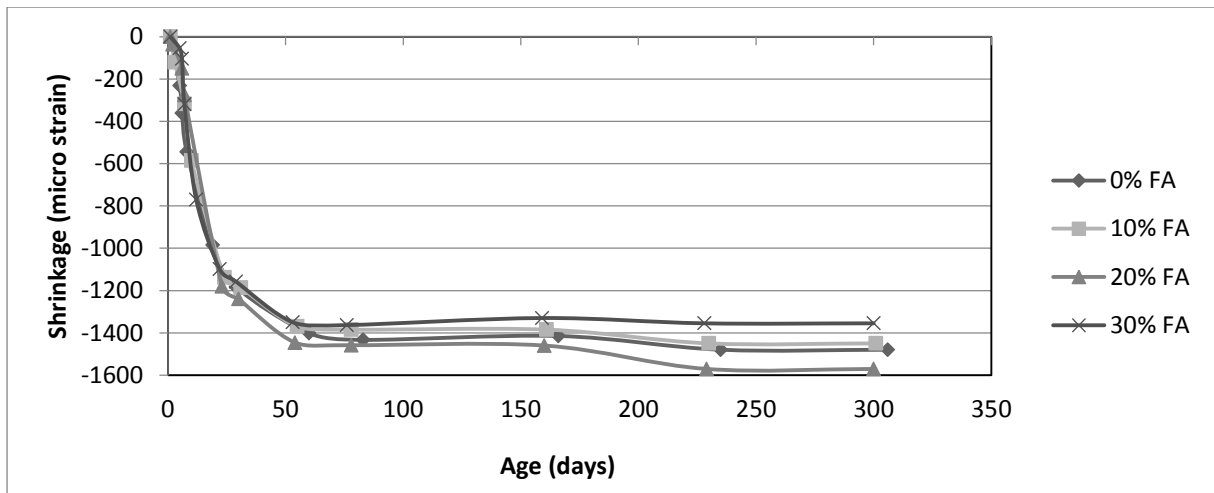
253 RSS demonstrated less drying shrinkage in comparison to those made with water.



254

255

Figure 12: Shrinkage of the mortar mixes with RSS and different u-FA content.



256
257 **Figure 13: Shrinkage of the mortar mixes with water and different u-FA content.**

258 4 Conclusions

- 259
- 260 • The addition of u-FA significantly reduced flowability for the mortar mixes with both
261 RSS and water. The flowability of the mortar mixes with RSS was comparatively less
262 than those with water.
 - 263 • For the mortar mixes with RSS and water, TWA generally increased with the inclusion
264 of u-FA. The TWA of the mortar mixes with RSS was comparatively less than those
265 with water.
 - 266 • For the mortar mixes with u-FA and RSS, the UPV values generally decreased with
267 the inclusion of u-FA at all curing ages. For the mortar mixes with water, the UPV
268 values at 1, 7 and 28 days decreased when the u-FA content increased and the
269 greatest UPV readings were recorded for the mortar mix with 0% u-FA. At 90 and
270 365 days, UPV increased with the inclusion of u-FA. UPV values of the mortar mixes
271 with RSS were comparatively less than those for the mixes with water.
 - 272 • For the mortar mixes that contained u-FA and RSS, the results showed that the
273 inclusion of 10-20% u-FA improved compressive strength at 28, 90 and 365 days. The
274 results also showed that the addition of u-FA improved long-term strength, and
prevented the decline in compressive strength observed for the mortar mix with RSS

275 and 0% u-FA at 365 days. For the mortar mixes with water, the compressive strength
276 at 1, 7, 28 and 90 day decreased when u-FA was included. However, the inclusion of
277 10% u-FA improved long-term compressive strength. Moreover, the compressive
278 strength of the mortar mixes with RSS was noticeably less than that of the mixes
279 with water.

- 280 • The addition of u-FA reduced drying shrinkage for the mortar mixes with both RSS
281 and water.

282 **5 Benefits and practical applications**

283 In addition to the production of sustainable construction materials, the outcome of utilising
284 sewage products in the construction industry could see huge financial savings to the current
285 economical constraints by eliminating the costly processes involved in treating these
286 wastes. This would also lead to a huge reduction in energy consumption. Furthermore,
287 there are huge environmental benefits from the prevention of RSS transportation to landfills
288 and incinerators. Using RSS as a water replacement in mortar or concrete mixes may
289 provide an opportunity to reduce the great demand on freshwater due to the continuous
290 and unsustainable growth in the world population.

291 Cement-based materials containing RSS and u-FA can be used in different construction and
292 civil engineering applications, and as follows:

- 293 • Masonry mortar for external applications
- 294 • In-situ concrete for external applications
- 295 • Precast units for external applications
- 296 • Self-compacting concrete
- 297 • Cement-based materials for road construction

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317 7 References

- 318 1. Metcalf & Eddy, I., et al., *Wastewater engineering : treatment, disposal, reuse*. 4th
319 ed. 2003, London: McGraw-Hill.
- 320 2. Waste on line. *Sewage Sludge*. 2010 12/11/2010]; Available from:
321 [http://www.wasteonline.org.uk/resources/Wasteguide/mn_wastetypes_sewageslud](http://www.wasteonline.org.uk/resources/Wasteguide/mn_wastetypes_sewagesludge.html/)
322 [ge.html/](http://www.wasteonline.org.uk/resources/Wasteguide/mn_wastetypes_sewagesludge.html/).
- 323 3. Defra, *Waste water treatment in the United Kingdom-2012-Implementation of the*
324 *European Union Urban Waste Water Treatment Directive-91/271/EEC*. 2012. p. 19.
- 325 4. Hamood, A. and J.M. Khatib, *Sustainability of sewage sludge in construction*, in
326 *Sustainability of Construction Materials*, J.M. Khatib, Editor. 2016, Woodhead
327 Publishing Series in Civil and Structural Engineering: United Kingdom. p. 625-641.
- 328 5. Monzó, J., et al., *Mechanical behavior of mortars containing sewage sludge ash (SSA)*
329 *and Portland cements with different tricalcium aluminate content*. Cement and
330 Concrete Research, 1999. **29**: p. 87-94.
- 331 6. Cyr, M., Marie Coutand, and P. Clastres, *Technological and environmental behavior*
332 *of sewage sludge ash (SSA) in cement-based materials*. Cement and Concrete
333 Research 2007. **37** p. 1278–1289.
- 334 7. Garcés, P., et al., *Mechanical and physical properties of cement blended with sewage*
335 *sludge ash*. Waste Management, 2008. **28**(12): p. 2495-2502.
- 336 8. Valls, S., *Leaching properties of stabilised/solidified cement-admixtures-sewage*
337 *sludges systems*. Waste Management, 2002. **22**: p. 37–45.
- 338 9. Cheilas, A., et al., *Impact of hardening conditions on to stabilized/solidified products*
339 *of cement–sewage sludge–jarosite/alunite*. Cement and Concrete Composites, 2007.
340 **29**(4): p. 263-269.
- 341 10. Yague, A., et al., *Durability of concrete with addition of dry sludge from waste water*
342 *treatment plants*. Cement and Concrete Research, 2005. **35**(6): p. 1064-1073.
- 343 11. Jordan, M., et al., *Application of sewage sludge in the manufacturing of ceramic tile*
344 *bodies*. Applied Clay Science, 2005. **30**(3-4): p. 219-224.
- 345 12. Favoni, C., et al., *Ceramic processing of municipal sewage sludge (MSS) and*
346 *steelworks slags (SS)*. Ceramics International, 2005. **31**(5): p. 697-702.
- 347 13. Montero, M.A., et al., *The use of sewage sludge and marble residues in the*
348 *manufacture of ceramic tile bodies*. Applied Clay Science, 2009. **46**(4): p. 404-408.
- 349 14. Zhao, Y., et al., *Research on sludge-fly ash ceramic particles (SFCP) for synthetic and*
350 *municipal wastewater treatment in biological aerated filter (BAF)*. Bioresource
351 Technology, 2009. **100**(21): p. 4955-4962.
- 352 15. Cusidó, J.A. and C. Soriano, *Valorization of pellets from municipal WWTP sludge in*
353 *lightweight clay ceramics*. Waste Management, 2011. **31**(6): p. 1372-1380.

- 354 16. Park, Y.J., S.O. Moon, and J. Heo, *Crystalline phase control of glass ceramics obtained*
355 *from sewage sludge fly ash*. *Ceramics International*, 2002. **29**: p. 223-227.
- 356 17. Merino, I., L.F. Arévalo, and F. Romero, *Preparation and characterization of ceramic*
357 *products by thermal treatment of sewage sludge ashes mixed with different*
358 *additives*. *Waste Management*, 2007. **27**(12): p. 1829-1844.
- 359 18. Chen, L. and D.F. Lin, *Applications of sewage sludge ash and nano-SiO₂ to*
360 *manufacture tile as construction material*. *Construction and Building Materials*, 2009.
361 **23**(11): p. 3312-3320.
- 362 19. Wang, K., et al., *Lightweight properties and pore structure of foamed material made*
363 *from sewage sludge ash*. *Construction and Building Materials*, 2005. **19**(8): p. 627-
364 633.
- 365 20. Chiou, I.-J., et al., *Lightweight aggregate made from sewage sludge and incinerated*
366 *ash*. *Waste Management*, 2006. **26**(12): p. 1453-1461.
- 367 21. Wang, X., et al., *Development of lightweight aggregate from dry sewage sludge and*
368 *coal ash*. *Waste Management*, 2009. **29**(4): p. 1330-1335.
- 369 22. Mun, K., *Development and tests of lightweight aggregate using sewage sludge for*
370 *nonstructural concrete*. *Construction and Building Materials*, 2007. **21**(7): p. 1583-
371 1588.
- 372 23. Cheeseman, C.R. and G.S. Viridi, *Properties and microstructure of lightweight*
373 *aggregate produced from sintered sewage sludge ash*. *Resources, Conservation and*
374 *Recycling*, 2005. **45**: p. 18–30.
- 375 24. Theodoratos, P., et al., *The use of municipal sewage sludge for the stabilization of*
376 *soil contaminated by mining activities*. *Journal of Hazardous Materials*, 2000. **B77**: p.
377 177-191.
- 378 25. Lin, D.-F., et al., *Sludge ash/hydrated lime on the geotechnical properties of soft soil*.
379 *Journal of Hazardous Materials*, 2007. **145**(1-2): p. 58-64.
- 380 26. Chen, L. and D.-F. Lin, *Stabilization treatment of soft subgrade soil by sewage sludge*
381 *ash and cement*. *Journal of Hazardous Materials*, 2009. **162**(1): p. 321-327.
- 382 27. Pan, S., C. Lin, and D. Tseng, *Reusing sewage sludge ash as adsorbent for copper*
383 *removal from wastewater*. *Resources, Conservation and Recycling*, 2003. **39**(1): p.
384 79-90.
- 385 28. Okol, R.E. and G. Balafoutas, *Landfill sealing potentials of bottom ashes of sludge*
386 *cakes*. *Soil & Tillage Research*, 1998. **46**: p. 307-314.
- 387 29. Sear, L.K.A., *Future trends for PFA in cementitious systems*. 1st Future Cement
388 Conference and Exhibition 2011-London Chamber of Commerce and Industry,
389 London UK, 2011.
- 390 30. Poon, C.S., X.C. Qiao, and Z.S. Lin, *Pozzolanic properties of reject fly ash in blended*
391 *cement pastes*. *Cement and Concrete Research*, 2003. **33**: p. 1857–1865.

- 392 31. Malhotra, V.M. and A.A. Ramezaniapour, *Fly ash in concrete*. 2nd ed. 1994, Canada:
393 CANMET.
- 394 32. ASTM, *ASTM C618-12a: Standard Specification for Coal Fly Ash and Raw or Calcined*
395 *Natural Pozzolan for Use in Concrete*. 2012.
- 396 33. BSI, *BS EN 450-1: Fly ash for concrete-Part 1: Definition, specifications and conformity*
397 *criteria*. 2007.
- 398 34. Snelson, D.G. and J.M. Kinuthia, *Resistance of mortar containing unprocessed*
399 *pulverised fuel ash (PFA) to sulphate attack*. *Cement and Concrete Composites*, 2010.
400 **32**: p. 523-531.
- 401 35. Snelson, D.G. and J.M. Kinuthia, *Characterisation of an unprocessed landfill ash for*
402 *application in concrete*. *Journal of Environmental Management*, 2010. **91**: p. 2117-
403 2125.
- 404 36. BSI, *BS EN 197-1: Cement-Part 1: Composition, specifications and conformity criteria*
405 *for common cements*. 2000.
- 406 37. British Lime Association. *Sewage sludge treatment*. 2013 05/03/2013]; Available
407 from: http://www.britishlime.org/technical/sewage_sludge_treatment.php.
- 408 38. BSI, *BS EN 12620:2002 +A1:2008: Aggregates for concrete*. 2008.
- 409 39. BSI, *BS EN 13139: Aggregates for mortar*. 2002.
- 410 40. BSI, *BS EN 1008: Mixing water for concrete-Specification for sampling, testing and*
411 *assessing the suitability of water, including water recovered from processes in the*
412 *concrete industry, as mixing water for concrete*. 2002.
- 413 41. BSI, *BS EN 206-1: Concrete-Part 1: Specification, performance, production and*
414 *conformity*. 2000.
- 415 42. BSI, *BS EN1097-3: Tests for mechanical and physical properties of aggregates-Part 3:*
416 *Determination of loose bulk density and voids*. 1998.
- 417 43. BSI, *BS EN 1097-7: Tests for mechanical and physical properties of aggregates-Part 7:*
418 *Determination of the particle density of filler -Pyknometer method*. 2008.
- 419 44. BSI, *BS EN 1015-3: Methods of test for mortar for masonry -Part 3: Determination of*
420 *consistence of fresh mortar (by flow table)*. 1999.
- 421 45. ASTM, *ASTM C109/C109M-08: Standard Test Method for Compressive Strength of*
422 *Hydraulic Cement Mortars (Using 2-in. or [50-mm] Cube Specimens)*. 2008.
- 423 46. Minnick, L.J., W.C. Webster, and E.J. Purdy, *Predictions of the fly effect of fly ash in*
424 *portland cement mortar and concrete*, in *Fly ash in concrete*, V.M. Malhotra and A.A.
425 Ramezaniapour, Editors. 1971, CANMET: Canada. p. Page 51.
- 426 47. Brink, R.H. and W.J. Halstead, *Studies relating to the testing of fly ash for use in*
427 *concrete*, in *Fly ash in concrete*, V.M. Malhotra and A.A. Ramezaniapour, Editors.
428 1956, CANMET: Canada. p. Page 51.

- 429 48. Welsh, G.B. and J.R. Burton, *Sydney fly ash in concrete*, in *Fly ash in concrete*, V.M.
430 Malhotra and A.A. Ramezaniapour, Editors. 1958, CANMET: Canada. p. Page 51.
- 431 49. Rehsi, S.S., *Studies on Indian fly ash and their use in structural concrete*, in *Fly ash in*
432 *concrete*, V.M. Malhotra and A.A. Ramezaniapour, Editors. 1973, CANMET: Canada.
433 p. Page 51.
- 434 50. Escalante-García, J.I. and J.H. Sharp, *The microstructure and mechanical properties of*
435 *blended cements hydrated at various temperatures*. Cement and Concrete Research,
436 2005. **31**(5): p. 695-702.
- 437 51. Kearsley, E.P. and P.J. Wainwright, *The effect of high fly ash content on the*
438 *compressive strength of foamed concrete*. Cement and Concrete Research, 2001. **31**:
439 p. 105-112.
- 440 52. Bouzoubaa, N., et al., *Laboratory-produced high-volume fly ash blended cements:*
441 *physical properties and compressive strength of mortars*. Cement and Concrete
442 Research, 1998. **28**(11): p. 1555-1569.
- 443 53. Poon, C.S., L. Lam, and Y.L. Wong, *A study on high strength concrete prepared with*
444 *large volumes of low calcium fly ash*. Cement and Concrete Research, 2000. **30**: p.
445 447-455.
- 446 54. Lam, L., Y.L. Wong, and C.S. Poon, *Effect of fly ash and silica fume on compressive*
447 *and fracture behaviors of concrete*. Cement and Concrete Research, 1998. **28**(2): p.
448 271–283.
- 449 55. Gebler, S.H. and P. Kleiger, *Effect of fly ash on physical properties of concrete*, in *Fly*
450 *ash in concrete*, V.M. Malhotra and A.A. Ramezaniapour, Editors. 1986, CANMET:
451 Canada. p. Page 77.
- 452 56. Tikalsky, P.J., P.M. Carrasquillo, and R.L. Carrasquillo, *Strength and durability*
453 *consideration affecting mix proportioning of concrete containing fly ash*, in *Fly ash in*
454 *concrete*, V.M. Malhotra and A.A. Ramezaniapour, Editors. 1988, CANMET: Canada.
455 p. Page 78.
- 456 57. Wild, S., B.B. Sabir, and J.M. Khatib, *Factors influencing strength development of*
457 *concrete containing silica fume*. Cement and Concrete Research, 1995. **25**(7): p.
458 1567-1580.

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